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FROM: Kipkoech Chepkoit, Ph.D., P.E.

DATE: 07/16/2019

SUBJECT: 18E0022A – Seismic Evaluation of Emery Pond

1 Introduction

The area currently occupied by Emery pond will be closed by removal of CCR and retrofitted to meet current Federal and State of Illinois regulations. The retrofitted pond will be designed to meet the liner criteria for new CCR surface impoundments of 40 CFR 257.72 and the structural integrity criteria of 40 CFR 257.74. The retrofit will require installation of a composite liner system meeting the federal requirements of 40 CFR 257.71. The retrofitted pond will be permitted and operated as a water treatment device under 35 IAC Section 309 Subpart B and also as a CCR surface impoundment under 40 CFR 257.

This memo presents seismic evaluation to document compliance with pertinent sections of 40 CFR 257 as stated above. The seismic impact zone is based on information obtained from geotechnical exploration performed in May 2019 and other existing reports.

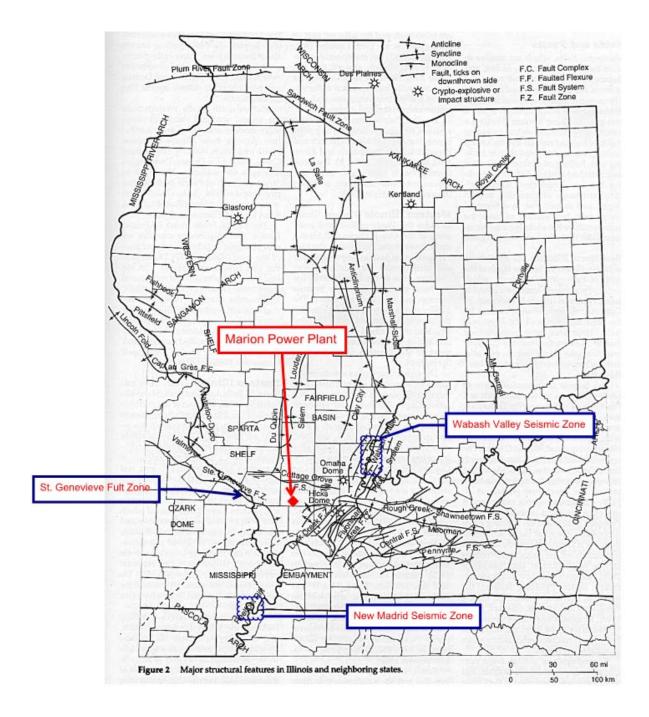
2 Site Seismicity

Marion power plant is located in an area of relatively high seismic activities. A record number of confirmed earthquakes of relatively high intensity have been recorded within 100 miles of the site. The figure below shows a map of all the fault zones within 100 mile radius of the project site.

The New Madrid fault zone, which is the primary region of seismic activity for the mid-continental region, is located approximately 80 miles southwest of Marion power plant. The fault zone in this area is characterized by high angle normal faults forming a complex horst and graben system. The strongest recorded earthquakes resulting from this fault zone occurred in December 1811 through February 1812, with three principal earthquakes of estimated Intensity XI on the Modified Mercalli Scale.

Another possible source of seismic activity is the St. Genevieve fault zone, which extends northwest/southeast from southwestern Illinois towards St. Genevieve County, Missouri. Several seismic events of body wave magnitude (m_b) 4.5 to 5.8 have been recorded near this fault zone.

The Wabash Valley Fault System is a tectonic region located in the Midwest of the United States, centered on the valley of the lower Wabash River, along southern Illinois and southwestern Indiana. This fault system is approximately 85 miles northeast of Marion power plant. The fault system consists of vertically oriented faults deeply buried under layer of sediment. This zone has been proven to have had earthquake for the last 20,000 years, with geologic evidence that they may have been as strong as 7.0–7.5 or greater on the Richter magnitude scale.



3 Evaluation Approach

The approach of evaluation is per MSHA Engineering Manual (Chapter 7) and existing literature. In brief the steps of approach are:

- Develop subsurface profile based on soil boring logs
- Characterize the subsurface material as "sand-like" or "clay-like" from field and laboratory testing
- Obtain earthquake ground parameters (USGS Earthquake Hazard Tools)
- Evaluate if the design earthquake will trigger a strength loss in sand-like or clay-like materials
- Evaluate seismic stability using the appropriate post-earthquake strength parameters
- Evaluate permanent deformations

4 Summary Design Soil Parameters

Table below presents summary of design soil parameters are based on visual description of the soils, field test results, lab test results and engineer's experience and knowledge of the site geology.

	Moist Unit	Undrained		Drai	ned	Material	
Material	Weight (pcf)	Cohesion (ksf)	Friction Angle (°)	Cohesion (ksf)	Friction Angle (°)	Characterization	
Ash Fill ¹	83	1.066			30	Clay-like	
Compacted Fill	120	1.0		0.05	30	Clay-like	
Structural Fill	120		30		30	Sand-like	
Clay/Clayey Silt ²	120	0.875	26.5	0.085	35	Clay-like	
Sandstone	130	5.0		5.00		NA	

¹Soil parameters based on triaxial UU test ²Soil parameters based on triaxial CU test

In addition the predominant soil material across the site, clay and/or clayey-silt had the following soil index parameters:

- Average fine content greater than 80%.
- Plasticity index varying from 10 to 15
- Liquidity index between -0.364 and 0.253

Given the subsurface material encountered, it is concluded the site is not prone to liquefaction.

5 Earthquake Design Parameters

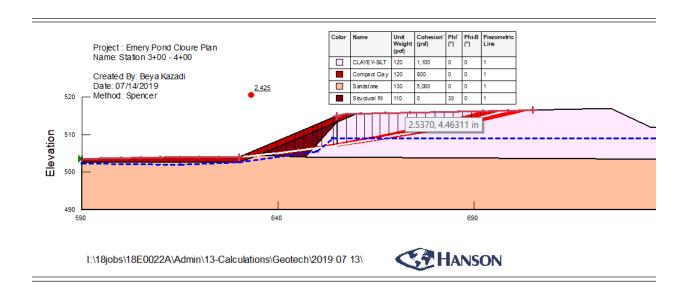
Seismic zones, which represent areas of the United States with the greatest seismic risk, are mapped by the U.S. Geological Survey (USGS) and readily available for the U.S. http://earthquake.usgs.gov/hazards/apps/) and commonly used to determine the maximum horizontal acceleration (MHA) in lithified earth material to evaluate if a site is located in a seismic impact zone.

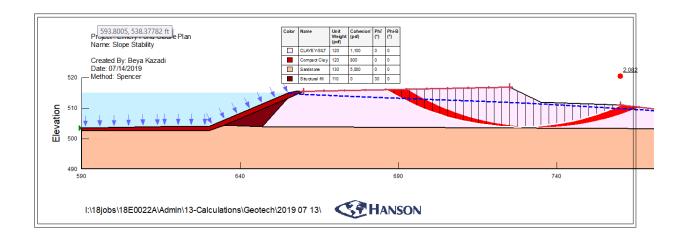
Using the site latitudes and longitudes in the USGS (2014) seismic hazard map tool, a Peak Ground Acceleration (PGA) of 0.507g is calculated at the B-C boundary (firm rock) in the USGS maps for 2475 years return period (98% or greater probability that the acceleration will not be exceeded in 50 years). This PGA is due to earthquake magnitude of 7.5. The USGS-generated MHA indicates that the site is within a seismic impact zone (>0.1 g). Therefore, the Emery Pond site should be considered to be in a seismic impact zone for this evaluation. PGA based on IBC 2015 was determined to be 0.584g from the USGS website. This takes into account Site Class D and maximum considered earthquake (MCE).

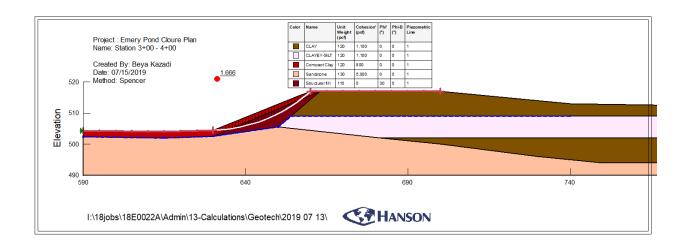
6 Slope Stability Results and Conclusion

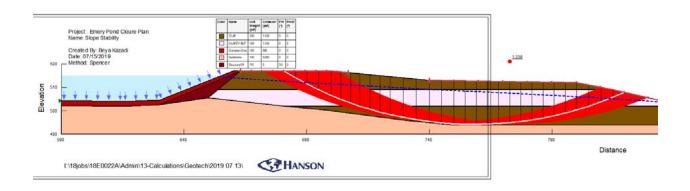
Post-earthquake slope stability analyses was performed with 80% peak undrained strength in Ash Fill, Compacted Clay, and Clay/Clayey-Silt. The strength parameters of the other layers remain the same based on SPT blow count assessment. Upstream and downstream slopes were analyzed. Upstream/impound slopes stability was analyzed assuming empty pond (critical condition) and downstream slope stability was analyzed assuming the pond is filled to El. 515. The upstream/impound slope results indicate factor of safety from 1.44 to 2.43. The downstream slope results indicate factor of safety from 1.23 to 2.08. These values exceed the required minimum seismic factor of safety of 1.2.

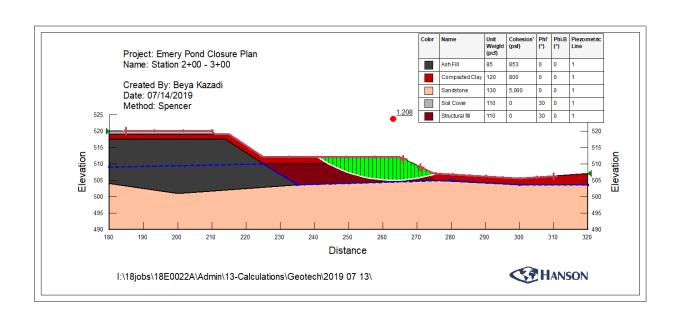
Given above post-earthquake factors of safety, deformations are considered acceptably small.











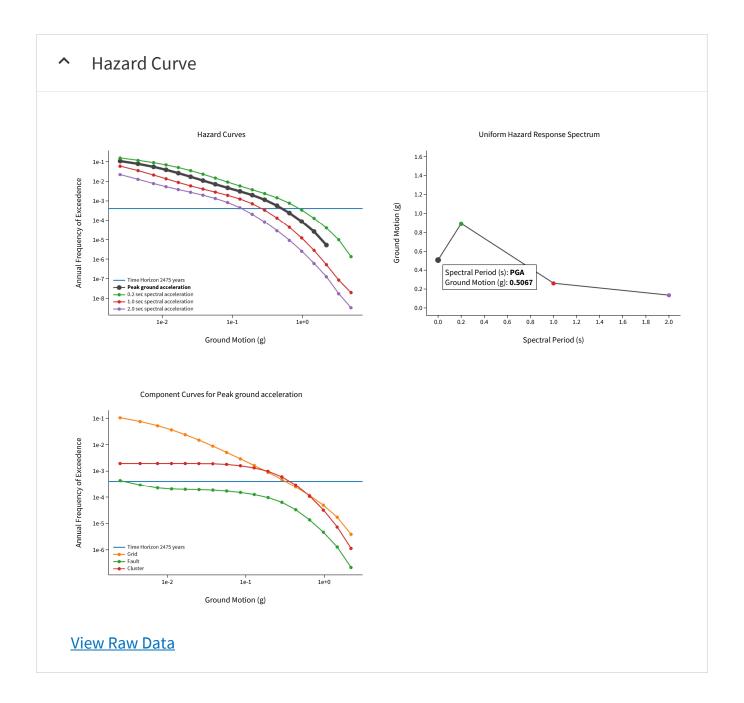
Unified Hazard Tool Page 1 of 7

Unified Hazard Tool

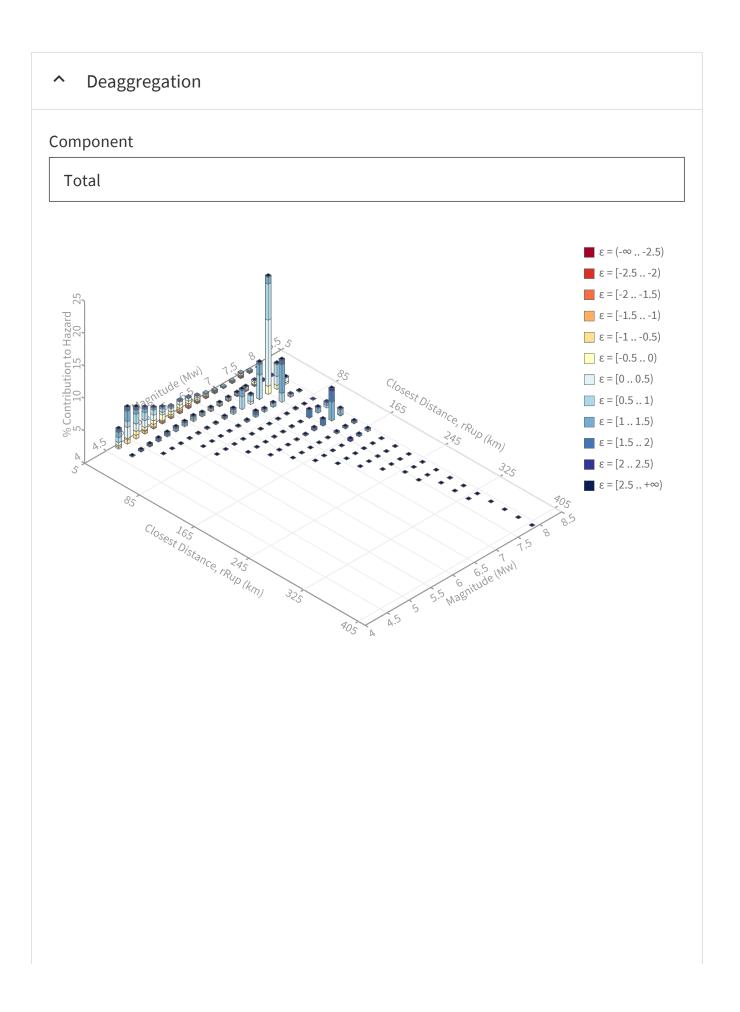
Please do not use this tool to obtain ground motion parameter values for the design code reference documents covered by the <u>U.S. Seismic Design Maps web tools</u> (e.g., the International Building Code and the ASCE 7 or 41 Standard). The values returned by the two applications are not identical.

Edition	Spectral Period
Dynamic: Conterminous U.S. 2014	Peak ground acceleration
Latitude	Time Horizon
Decimal degrees	Return period in years
37.618512	2475
Longitude	
Decimal degrees, negative values for western long	
-88.953743	
Site Class	
760 m/s (B/C boundary)	

Unified Hazard Tool Page 2 of 7



Unified Hazard Tool Page 3 of 7



Unified Hazard Tool Page 4 of 7

Summary statistics for, Deaggregation: Total

Deaggregation targets

Return period: 2475 yrs

Exceedance rate: 0.0004040404 yr⁻¹ **PGA ground motion:** 0.50670301 g

Recovered targets

Return period: 2482.7409 yrs

Exceedance rate: 0.00040278066 yr⁻¹

Totals

Binned: 100 % Residual: 0 % Trace: 0.56 %

Mean (for all sources)

r: 46.75 km **m:** 6.73 **ε₀:** 0.65 σ

Mode (largest r-m bin)

r: 44.26 km m: 7.52 ε₀: 0.4 σ

Contribution: 18.17 %

Mode (largest ε₀ bin)

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Unified Hazard Tool Page 6 of 7

Deaggregation Contributors

Source Set ↳ Source	Туре	r	m	ε ₀	lon	lat	az	%
SSCn New Madrid	Cluster							31.10
NMFS RLME 4		53.11	7.45	0.54	89.020°W	37.270°N	188.60	7.83
NMFS RLME 1		101.48	7.58	1.36	89.288°W	36.995°N	203.19	5.49
NMFS RLME 7		51.59	7.44	0.52	89.020°W	37.270°N	188.60	5.11
NMFS RLME 3		53.10	7.45	0.54	89.020°W	37.270°N	188.60	3.35
NMFS RLME 5		101.24	7.56	1.37	89.288°W	36.995°N	203.19	3.34
NMFS RLME 2		101.49	7.58	1.36	89.288°W	36.995°N	203.19	2.35
NMFS RLME 8		51.58	7.44	0.52	89.020°W	37.270°N	188.60	2.19
NMFS RLME 6		101.25	7.56	1.37	89.288°W	36.995°N	203.19	1.43
SSCn Fixed Smoothing Zone 6 (opt)	Grid							15.45
PointSourceFinite: -88.954, 37.731		12.75	5.51	0.46	88.954°W	37.731°N	0.00	4.29
PointSourceFinite: -88.954, 37.686		8.75	5.33	0.12	88.954°W	37.686°N	0.00	3.62
PointSourceFinite: -88.954, 37.641		5.56	5.23	-0.30	88.954°W	37.641°N	0.00	3.13
PointSourceFinite: -88.954, 37.776		16.82	5.73	0.66	88.954°W	37.776°N	0.00	1.51
PointSourceFinite: -88.954, 37.821		20.83	5.95	0.76	88.954°W	37.821°N	0.00	1.26
USGS Fixed Smoothing Zone 1 (opt)	Grid							14.62
PointSourceFinite: -88.954, 37.731		12.81	5.49	0.49	88.954°W	37.731°N	0.00	4.23
PointSourceFinite: -88.954, 37.686		8.77	5.31	0.13	88.954°W	37.686°N	0.00	3.59
PointSourceFinite: -88.954, 37.641		5.56	5.22	-0.29	88.954°W	37.641°N	0.00	3.11
PointSourceFinite: -88.954, 37.821		21.05	5.90	0.84	88.954°W	37.821°N	0.00	1.29
PointSourceFinite: -88.954, 37.776		16.94	5.69	0.71	88.954°W	37.776°N	0.00	1.07
USGS New Madrid 500-year	Cluster							9.61
NMSZ: Center Model		68.90	7.49	0.98	89.070°W	37.165°N	191.55	7.04
NMSZ: Mid-West Model		65.59	7.48	0.92	89.193°W	37.218°N	205.45	1.09
USGS New Madrid 750-year	Cluster							6.41
NMSZ: Center Model		68.90	7.49	0.98	89.070°W	37.165°N	191.55	4.69
USGS New Madrid 500-year	Fault							5.72
New Madrid central		52.43	7.62	0.61	89.070°W	37.165°N	191.55	4.23
SSCn Adaptive Smoothing Zone 6 (opt)	Grid							3.65
USGS Adaptive Smoothing Zone 1 (opt)	Grid							3.30
Commerce Lineament	Grid							3.15
USGS Fixed Smoothing Zone 2 (opt)	Grid							1.75
Wabash Valley	Grid							1.42
SSCn Fixed Smoothing Zone 8 (opt)	Grid							1.13





Latitude, Longitude: 37.618512, -88.953743

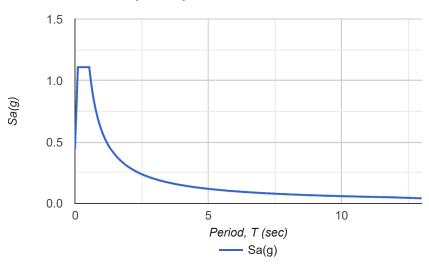


Date	7/16/2019, 5:48:54 PM
Design Code Reference Document	IBC-2015
Risk Category	
Site Class	D - Stiff Soil

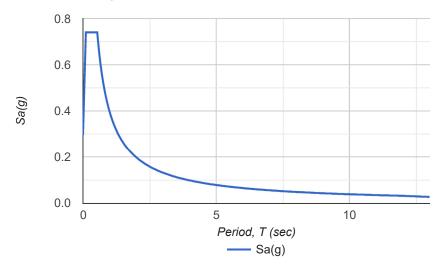
Туре	Value	Description
Ss	1.013	MCE _R ground motion. (for 0.2 second period)
S ₁	0.349	MCE _R ground motion. (for 1.0s period)
S _{MS}	1.109	Site-modified spectral acceleration value
S _{M1}	0.594	Site-modified spectral acceleration value
S _{DS}	0.74	Numeric seismic design value at 0.2 second SA
S _{D1}	0.396	Numeric seismic design value at 1.0 second SA

Туре	Value	Description
SDC	D	Seismic design category
Fa	1.095	Site amplification factor at 0.2 second
F _v	1.703	Site amplification factor at 1.0 second
PGA	0.584	MCE _G peak ground acceleration
F _{PGA}	1	Site amplification factor at PGA
PGA _M	0.584	Site modified peak ground acceleration
TL	12	Long-period transition period in seconds
SsRT	1.013	Probabilistic risk-targeted ground motion. (0.2 second)
SsUH	1.222	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration
SsD	1.5	Factored deterministic acceleration value. (0.2 second)
S1RT	0.349	Probabilistic risk-targeted ground motion. (1.0 second)
S1UH	0.43	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration.
S1D	0.6	Factored deterministic acceleration value. (1.0 second)
PGAd	0.6	Factored deterministic acceleration value. (Peak Ground Acceleration)
C _{RS}	0.829	Mapped value of the risk coefficient at short periods
C _{R1}	0.811	Mapped value of the risk coefficient at a period of 1 s

MCER Response Spectrum



Design Response Spectrum



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Latitude, Longitude: 37.618512, -88.953743

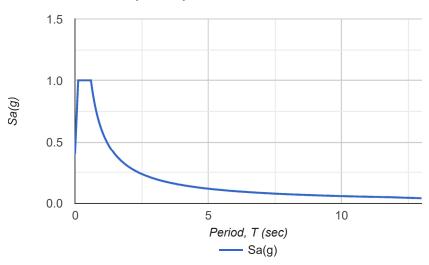


Date	7/16/2019, 5:50:14 PM
Design Code Reference Document	NEHRP-2015
Risk Category	
Site Class	D - Stiff Soil

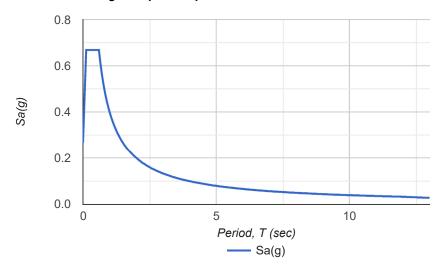
Туре	Value	Description
Ss	0.869	MCE _R ground motion. (for 0.2 second period)
S ₁	0.299	MCE _R ground motion. (for 1.0s period)
S _{MS}	1.002	Site-modified spectral acceleration value
S _{M1}	0.599 -See Section 11.4.7	Site-modified spectral acceleration value
S _{DS}	0.668	Numeric seismic design value at 0.2 second SA
S _{D1}	0.399 -See Section 11.4.7	Numeric seismic design value at 1.0 second SA

Туре	Value	Description
SDC	D -See Section 11.4.7	Seismic design category
Fa	1.152	Site amplification factor at 0.2 second
F _v	2.002 -See Section 11.4.7	Site amplification factor at 1.0 second
PGA	0.516	MCE _G peak ground acceleration
F _{PGA}	1.1	Site amplification factor at PGA
PGA _M	0.567	Site modified peak ground acceleration
TL	12	Long-period transition period in seconds
SsRT	0.869	Probabilistic risk-targeted ground motion. (0.2 second)
SsUH	0.996	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration
SsD	2.905	Factored deterministic acceleration value. (0.2 second)
S1RT	0.299	Probabilistic risk-targeted ground motion. (1.0 second)
S1UH	0.344	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration.
S1D	0.92	Factored deterministic acceleration value. (1.0 second)
PGAd	1.539	Factored deterministic acceleration value. (Peak Ground Acceleration)
C _{RS}	0.873	Mapped value of the risk coefficient at short periods
C _{R1}	0.868	Mapped value of the risk coefficient at a period of 1 s

MCER Response Spectrum



Design Response Spectrum



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